



Stantec Consulting Services Inc.
3052 Beaumont Centre Circle, Lexington KY 40513

March 26, 2021
File: rpt_002_let_175666001
Revision 0

Tennessee Valley Authority
1101 Market Street
Chattanooga, Tennessee 37402

**RE: Initial Run-On and Run-Off Control System Plan
New CCR Landfill
TVA Shawnee Fossil Plant
Paducah, McCracken County, Kentucky**

1.0 PURPOSE

As described in 40 CFR § 257.81, an owner or operator of a new CCR landfill is required to demonstrate that run-on and run-off control systems have been designed and constructed to meet the EPA Final CCR Rule. This letter documents Stantec's certification that the Initial Run-On and Run-Off Plan for the TVA Shawnee Fossil Plant's (SHF) New CCR Landfill complies with requirements in the EPA Final CCR Rule 40 CFR § 257.81.

2.0 SUMMARY OF FINDINGS

The attached plan documents that the run-on and run-off control system requirements for the New CCR Landfill meet the requirements set forth in 40 CFR § 257.81.

3.0 QUALIFIED PROFESSIONAL ENGINEER CERTIFICATION

I, Michael J. Steele, being a Professional Engineer in good standing in the Commonwealth of Kentucky, do hereby certify, to the best of my knowledge, information, and belief:

1. that the information contained in this certification is prepared in accordance with the accepted practice of engineering;
2. that the information contained herein is accurate as of the date of my signature below; and
3. that the Initial Run-On and Run-Off Control System Plan for the TVA SHF New CCR Landfill meets the requirements specified in 40 CFR § 257.81.



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Page 2 of 2

**RE: Initial Run-On and Run-Off Control System Plan
New CCR Landfill
TVA Shawnee Fossil Plant
Paducah, McCracken County, Kentucky**

SIGNATURE  _____

DATE 3/26/2021

ADDRESS: Stantec Consulting Services Inc.
3052 Beaumont Centre Circle
Lexington, Kentucky 40513

TELEPHONE: (859) 422-3000

ATTACHMENTS: Initial Run-On and Run-Off Control System Plan



**Initial Run-On and Run-Off
Control System Plan
TVA Shawnee New CCR
Landfill**

New CCR Landfill
TVA Shawnee Fossil Plant
Paducah, McCracken County,
Kentucky



Prepared for:
Tennessee Valley Authority
Chattanooga, Tennessee

Prepared by:
Stantec Consulting Services Inc.
Lexington, Kentucky

March 26, 2021
Revision 0

**INITIAL RUN-ON AND RUN-OFF CONTROL SYSTEM PLAN
TVA SHAWNEE NEW CCR LANDFILL**

March 26, 2021

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INITIAL RUN-ON AND RUN-OFF CONTROL SYSTEM PLAN TVA SHAWNEE NEW CCR LANDFILL

March 26, 2021

1.0 INTRODUCTION

On April 17, 2015, the Environmental Protection Agency (EPA) published the "Disposal of Coal Combustion Residuals (CCR) from Electric Utilities" final rule (EPA Final CCR Rule) in the Federal Register. The Tennessee Valley Authority (TVA) contracted with Stantec Consulting Services Inc. (Stantec) to provide an initial run-on and run-off control system plan for the Shawnee Fossil Plant's (SHF) New CCR Landfill that meets requirements of the EPA Final CCR Rule § 257.81.

1.1 OBJECTIVE

As required by § 257.81 of the EPA Final CCR Rule, an owner or operator of a new CCR landfill is required to demonstrate that run-on and run-off control systems have been designed and constructed to meet the CCR Rule. The objective of this plan is to document that these systems comply with the requirements.

1.2 UNIT DESCRIPTION

SHF is a coal-fired, electric-generating plant. The plant is located in McCracken County, Kentucky, along the south shore of the Ohio River near river mile 946, just east of the confluence of Little Bayou Creek with the Ohio River.

The New CCR Landfill will be located on the Shawnee East Site, which consists of about 205 acres that TVA acquired in 2016 next to the eastern boundary of the SHF reservation. The landfill will be constructed in three stages over a total footprint of 88 acres. The embankment will be about 115 feet tall with maximum 4H:1V slopes and will accommodate about 8 million cubic yards of CCR material (fly ash, bottom ash, and gypsum) across an estimated 25-year operational life.

2.0 CRITERIA

The EPA Final CCR Rule § 257.81 requirements for the run-on and run-off control system are:

40 CFR § 257.81(a). *The owner or operator of an existing or new CCR landfill or any lateral expansion of a CCR landfill must design, construct, operate and maintain:*

- (1) *A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm; and*
- (2) *A run-off control system from the active portion of the CCR unit to collect and control at least the water volume resulting from a 24-hour, 25-year storm.*

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40 CFR § 257.81(b). *Run-off from the active portion of the CCR unit must be handled in accordance with the surface water requirements under § 257.3-3.*

40 CFR § 257.81(c)(1). *The owner or operator must prepare initial and periodic run-on and run-off control system plans for the CCR unit according to the timeframes specified in paragraphs (c) (3) and (4) of this section. These plans must document how the run-on and run-off control systems have been designed and constructed to meet the applicable requirements of this section. Each plan must be supported by appropriate engineering calculations.*

40 CFR § 257.81(c)(3)(ii). *The owner or operator must prepare the initial run-on and run-off control system plan no later than the date of initial receipt of CCR in the CCR unit.*

3.0 INITIAL RUN-ON AND RUN-OFF CONTROL SYSTEM PLAN

This plan describes how the New CCR Landfill will meet requirements of § 257.81 of the EPA Final CCR Rule. It is noted that the landfill will not have run-on flow as it will be constructed to elevations above adjacent ground.

The designed run-off controls for the New CCR Landfill will consist of ditches/flumes, piping, and ponds. A series of flumes and bench ditches will convey surface water from the landfill to a perimeter ditch that follows the base of the embankment. Within the perimeter ditch, drop inlet structures will collect and convey flow to an underground pipe system that will generally follow the alignment of the perimeter ditch. The pipe system will flow by gravity to one of two stormwater ponds, which will serve as sediment control structures and ultimately discharge into an unnamed tributary of Little Bayou Creek. The pond discharge will be monitored through a Kentucky Pollutant Discharge Elimination System (KPDES) compliance point.

Criteria listed in Section 2.0 was used to design the system described above. PC-SWMM modeling software (version 5.1) was used to model the hydrology and hydraulics. The model consists of subwatersheds to simulate run-off over a specified area, inlets collecting the run-off, pipes conveying the run-off downstream, and hydraulic outlet structures which store and release the run-off.

Details of the run-off hydrology and hydraulics are provided in the following sections.

3.1 HYDROLOGIC ANALYSIS

The hydrology of the site was modeled using Natural Resources Conservation Service (NRCS) methodologies within PC-SWMM. Standard values were selected from tables provided with the *User's Guide to SWMM 5 (13th Edition)* for parameters not discussed below.

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3.1.1 Rainfall Run-off and Distribution

The precipitation depth for the 24-hour, 25-year storm is 6.1 inches and was obtained from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14. The NOAA precipitation data is provided in Appendix A. The SCS Type II storm distribution was applied to develop a rainfall hyetograph.

3.1.2 Curve Number

The Curve Number method was used to model stormwater infiltration. For the closed/capped condition, the cover type was assumed to be "open space (lawns, parks, etc.)". The design final cover system will consist of (from bottom to top) a minimum 6-inch soil base, 40-mil thick textured Linear Low Density Polyethylene (LLDPE) flexible geomembrane, a geocomposite drainage layer, 18 inches of protective cover soil, and 6 inches of vegetative cover soil. A soil group D classification was assumed for the cover. This yields a curve number of 80/84/89 for Good/Fair/Poor (respectively) grass cover conditions. A "good condition (grass cover > 75%)" has been assumed. Therefore, the curve number used in modeling capped landfill areas was 80.

3.1.3 Subwatershed Delineation

Subwatersheds were delineated based on the landfill surface.

3.1.4 Lag Time

Each subwatershed was assigned an average slope percentage, determined from the landfill surface. Flow lengths were also calculated for each subwatershed as the sum of the sheet flow, shallow concentrated flow, and channel flow.

3.1.5 Reach Routing

The dynamic wave method was the selected routing method. The dynamic wave method in PC-SWMM allows for channel storage, backwater, entrance/exit losses, flow reversal, and pressurized flow. This routing represents pressurized flow when a closed conduit is full and allows flooding to occur when the water depth exceeds the maximum available depth at an inlet.

3.2 HYDRAULIC ANALYSIS

Hydraulic calculations were performed to design the bench ditches, flumes, perimeter ditch, pipe network, and stormwater ponds as described below.

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3.2.1 Bench Ditches and Flumes

The bench ditches will be v-shaped with a right side-slope of 10H:1V, a left side-slope of 4H:1V, a depth of 2.5 feet, and a channel slope of 1-2%. A Manning's roughness (n) of 0.033 was assumed based on a vegetated condition. PC-SWMM was used to estimate the peak run-off for the 24-hour, 25-year storm for the drainage area associated with each bench ditch. The peak run-off associated with the largest subwatershed area was used to check the capacity of the bench ditches using Manning's equation for open channel flow.

The flumes will be trapezoidal in shape with a 6-foot bottom width, 3H:1V side slopes, a depth of 2 feet, and a channel slope of 25%. A Manning's roughness (n) of 0.04 was assumed for the armored flumes. PC-SWMM was used to estimate the peak run-off for the 24-hour, 25-year storm for the drainage area associated with each flume. The peak run-off associated with the largest subwatershed was used to check the capacity of the flumes.

3.2.2 Pipes and Perimeter Ditch

The perimeter ditch and pipe network were designed using PC-SWMM.

The perimeter ditch will serve as storage above the rim of the drop inlet structures. Each inlet was modeled as a storage node within PC-SWMM to account for this storage. Stage-area curves were computed using the landfill surface and were computed from the rim of the drop inlet structure to the crest of the perimeter road (which represents the top of the ditch). The tabular stage-area curves are included in the PC-SWMM input in Appendix B.

3.2.3 Stormwater Ponds

Two stormwater ponds were designed to collect and control the stormwater from the landfill prior to discharging it to an unnamed tributary of Little Bayou Creek. Each pond will include a perforated primary spillway, and an emergency spillway. The low flow outlets will be located on the riser to reduce exit velocities during smaller storm events, thus reducing the amount of sediment transported from the site. The first set of perforations will consist of six, 1.5-inch diameter orifices set at elevation 354 feet. After that, there will be additional rows consisting of six, 1-inch diameter perforations placed at 1-foot intervals from elevation 355 feet to elevation 358 feet. These perforations are used to set the normal pool elevation. The first row of six, 1.5-inch diameter orifices are modeled in PC-SWMM to allow the ponds to accurately portray the normal pool elevation. The primary spillway will consist of a 24-inch diameter high-density polyethylene (HDPE) riser structure with a top elevation of 359 feet. The risers will be connected to horizontal 24-inch diameter HDPE pipes. This vertical riser was modeled in PC-SWMM as a bottom orifice connected to a pipe.

The emergency spillway will be trapezoidal in shape with a 10-foot bottom width, 2-foot height, 5H:1V side-slopes, and placed at an invert elevation of 362 feet. The emergency spillway was modeled as a trapezoidal weir.

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3.3 RESULTS

Results are shown in Appendix C for the bench ditches and flumes, and Appendix D for the pipe network.

The peak run-off associated with the bench ditches was 5.9 cubic feet per second (cfs). The maximum depth was determined to be 0.6 feet for the 24-hour, 25-year storm. This results in a minimum of 1.9 feet of freeboard.

The peak run-off associated with the flumes was 27.5 cfs. The maximum depth was determined to be 0.5 feet for the 24-hour, 25-year storm. This results in a minimum of 1.5 feet of freeboard.

The pipe network profiles from PC-SWMM were used to check the elevations within the perimeter ditch for the 24-hour, 25-year storm. The profiles show that the water surface elevation is below the top of the road elevation, with a minimum of 2.9 feet of freeboard.

The peak 24-hour, 25-year water surface elevation within the ponds is 359.6 and 359.7 for the north and south stormwater ponds, respectively. This results in a freeboard of 4.4 feet for the north stormwater pond and 4.3 feet for the south stormwater pond.

3.4 CONSTRUCTION

The Stage 1 stormwater pipes and perimeter ditch and facility stormwater pond have been constructed in accordance with the design. Temporary benches and flumes will be constructed as embankment placement reaches design bench elevations and temporary cover is placed on the outslopes. During closure of the landfill cell (Stage 1), permanent flumes will be constructed on top of the final cover system.

3.5 SUMMARY

The following summarizes compliance with EPA Final CCR Rule criteria:

1. Run-on: The landfill will not have run-on flow as it will be constructed to elevations above adjacent ground.
2. Run-off: The bench ditches, flumes, pipe network, perimeter ditch, and stormwater ponds are capable of collecting and controlling the run-off water volume of a 24-hour, 25-year storm. The closed condition of the landfill was evaluated for design purposes. The landfill will be constructed in stages, with run-off increasing as each subsequent stage is constructed. The results presented above show that the landfill will be capable of conveying the run-off associated with the closed condition, and therefore will also be capable of conveying the run-off for interim conditions.

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3. Run-off (permitted discharge): Run-off from the landfill flows to the stormwater ponds. The pond discharge will be monitored through a KPDES compliance point. Therefore, the run-off is handled in accordance with the surface water requirements.

3.6 INITIAL RUN-ON AND RUN-OFF CONTROL SYSTEM PLAN

This document constitutes the Initial Run-On and Run-Off Control System Plan as required by § 257.81(c).

3.7 AMENDMENTS TO PLAN

If there is a change in conditions that would substantially affect the current version of the Initial Run-on and Run-off Control System Plan, then the plan will be amended as required by § 257.81(c)(2) and placed in the operating record as required by § 257.105(g)(3).

4.0 CONCLUSION

The Initial Run-on and Run-off Control System Plan for the TVA SHF New CCR Landfill meets the requirements of § 257.81 of the EPA Final CCR Rule.

5.0 REFERENCES

James, et. al., User's Guide to SWMM5, 13th Edition, November, 2010.

NOAA Atlas 14, Precipitation Frequency Atlas of the United States, Volume 2, Version 3, 2006.

APPENDIX A
NOAA RAINFALL DEPTHS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerals](#)

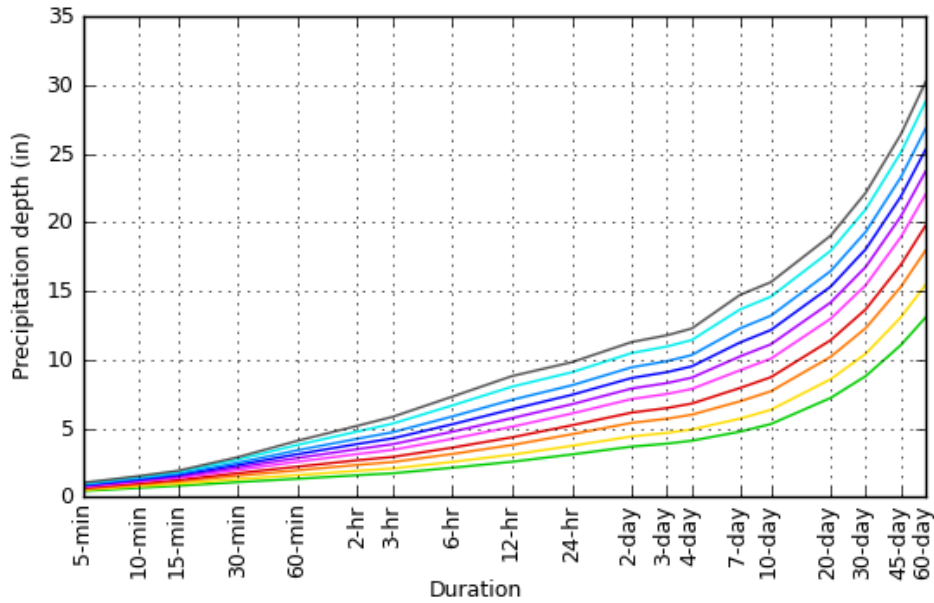
PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.421 (0.388-0.458)	0.496 (0.458-0.540)	0.577 (0.531-0.627)	0.641 (0.590-0.696)	0.721 (0.660-0.782)	0.779 (0.712-0.844)	0.837 (0.761-0.905)	0.897 (0.812-0.970)	0.973 (0.875-1.05)	1.03 (0.921-1.12)
10-min	0.657 (0.605-0.715)	0.779 (0.718-0.847)	0.902 (0.830-0.980)	0.996 (0.916-1.08)	1.11 (1.02-1.20)	1.19 (1.09-1.29)	1.27 (1.16-1.38)	1.35 (1.22-1.46)	1.45 (1.30-1.57)	1.52 (1.36-1.65)
15-min	0.807 (0.744-0.879)	0.956 (0.882-1.04)	1.11 (1.03-1.21)	1.23 (1.13-1.34)	1.38 (1.26-1.49)	1.48 (1.35-1.60)	1.59 (1.44-1.72)	1.69 (1.52-1.82)	1.81 (1.63-1.96)	1.90 (1.70-2.06)
30-min	1.08 (0.991-1.17)	1.29 (1.19-1.40)	1.54 (1.42-1.67)	1.73 (1.59-1.87)	1.96 (1.80-2.13)	2.14 (1.96-2.32)	2.31 (2.10-2.50)	2.49 (2.25-2.69)	2.72 (2.45-2.94)	2.89 (2.58-3.13)
60-min	1.32 (1.22-1.44)	1.59 (1.46-1.73)	1.94 (1.78-2.10)	2.20 (2.03-2.39)	2.56 (2.34-2.77)	2.83 (2.59-3.07)	3.11 (2.83-3.37)	3.40 (3.08-3.67)	3.79 (3.41-4.10)	4.09 (3.65-4.43)
2-hr	1.58 (1.45-1.72)	1.90 (1.74-2.07)	2.33 (2.13-2.54)	2.67 (2.44-2.90)	3.12 (2.84-3.39)	3.48 (3.15-3.78)	3.85 (3.47-4.19)	4.23 (3.80-4.60)	4.76 (4.24-5.18)	5.18 (4.58-5.64)
3-hr	1.72 (1.57-1.88)	2.06 (1.89-2.27)	2.53 (2.32-2.78)	2.91 (2.65-3.19)	3.42 (3.11-3.74)	3.83 (3.47-4.18)	4.26 (3.84-4.64)	4.70 (4.21-5.13)	5.33 (4.73-5.81)	5.83 (5.13-6.35)
6-hr	2.12 (1.94-2.34)	2.55 (2.33-2.81)	3.13 (2.86-3.44)	3.59 (3.27-3.94)	4.23 (3.83-4.63)	4.74 (4.28-5.19)	5.29 (4.74-5.78)	5.86 (5.22-6.40)	6.66 (5.88-7.28)	7.30 (6.40-7.99)
12-hr	2.56 (2.34-2.81)	3.08 (2.81-3.38)	3.78 (3.45-4.14)	4.34 (3.95-4.76)	5.11 (4.63-5.59)	5.74 (5.18-6.27)	6.39 (5.72-6.98)	7.07 (6.30-7.73)	8.03 (7.08-8.80)	8.80 (7.70-9.66)
24-hr	3.09 (2.88-3.32)	3.72 (3.47-3.99)	4.57 (4.25-4.90)	5.21 (4.85-5.59)	6.07 (5.64-6.51)	6.75 (6.24-7.23)	7.43 (6.86-7.96)	8.13 (7.47-8.71)	9.08 (8.30-9.73)	9.81 (8.93-10.5)
2-day	3.66 (3.41-3.93)	4.40 (4.10-4.73)	5.39 (5.02-5.78)	6.14 (5.71-6.58)	7.13 (6.61-7.63)	7.89 (7.31-8.45)	8.66 (8.00-9.28)	9.44 (8.69-10.1)	10.5 (9.61-11.2)	11.3 (10.3-12.1)
3-day	3.88 (3.61-4.16)	4.66 (4.34-5.00)	5.69 (5.30-6.10)	6.47 (6.01-6.93)	7.49 (6.95-8.03)	8.29 (7.67-8.88)	9.08 (8.38-9.73)	9.88 (9.10-10.6)	10.9 (10.0-11.8)	11.8 (10.7-12.7)
4-day	4.09 (3.81-4.39)	4.91 (4.57-5.27)	5.99 (5.58-6.42)	6.79 (6.32-7.29)	7.86 (7.29-8.43)	8.68 (8.04-9.30)	9.50 (8.77-10.2)	10.3 (9.50-11.1)	11.4 (10.5-12.3)	12.3 (11.2-13.2)
7-day	4.75 (4.42-5.11)	5.70 (5.30-6.13)	6.96 (6.47-7.48)	7.93 (7.35-8.52)	9.22 (8.53-9.91)	10.2 (9.44-11.0)	11.2 (10.3-12.1)	12.3 (11.2-13.2)	13.6 (12.4-14.7)	14.7 (13.4-15.9)
10-day	5.30 (4.94-5.69)	6.34 (5.91-6.81)	7.69 (7.17-8.26)	8.71 (8.12-9.35)	10.1 (9.35-10.8)	11.1 (10.3-11.9)	12.1 (11.2-13.1)	13.2 (12.2-14.2)	14.6 (13.4-15.7)	15.6 (14.3-16.8)
20-day	7.21 (6.76-7.67)	8.57 (8.05-9.14)	10.2 (9.57-10.9)	11.4 (10.7-12.2)	13.0 (12.1-13.8)	14.2 (13.2-15.1)	15.3 (14.3-16.3)	16.4 (15.3-17.5)	17.9 (16.6-19.1)	19.0 (17.6-20.3)
30-day	8.80 (8.29-9.35)	10.4 (9.84-11.1)	12.3 (11.6-13.1)	13.7 (12.9-14.5)	15.4 (14.5-16.4)	16.7 (15.7-17.8)	18.0 (16.9-19.2)	19.3 (18.0-20.5)	20.9 (19.5-22.3)	22.1 (20.6-23.6)
45-day	11.0 (10.4-11.7)	13.1 (12.3-13.9)	15.3 (14.4-16.2)	16.9 (15.8-17.9)	18.9 (17.7-20.0)	20.4 (19.1-21.6)	21.9 (20.4-23.2)	23.3 (21.7-24.7)	25.1 (23.3-26.6)	26.4 (24.4-28.1)
60-day	13.1 (12.3-13.8)	15.4 (14.6-16.3)	17.9 (17.0-18.9)	19.8 (18.7-20.8)	22.0 (20.8-23.2)	23.7 (22.3-25.0)	25.3 (23.8-26.7)	26.8 (25.2-28.3)	28.8 (27.0-30.5)	30.2 (28.2-32.0)

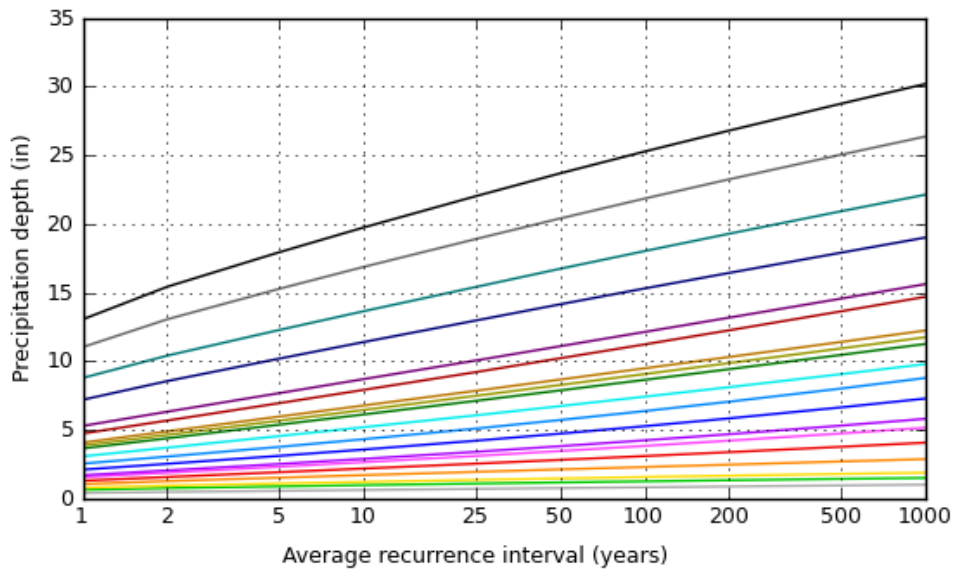
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

PF graphical

PDS-based depth-duration-frequency (DDF) curves
 Latitude: 37.1399°, Longitude: -88.7742°



Average recurrence interval (years)
1
2
5
10
25
50
100
200
500
1000

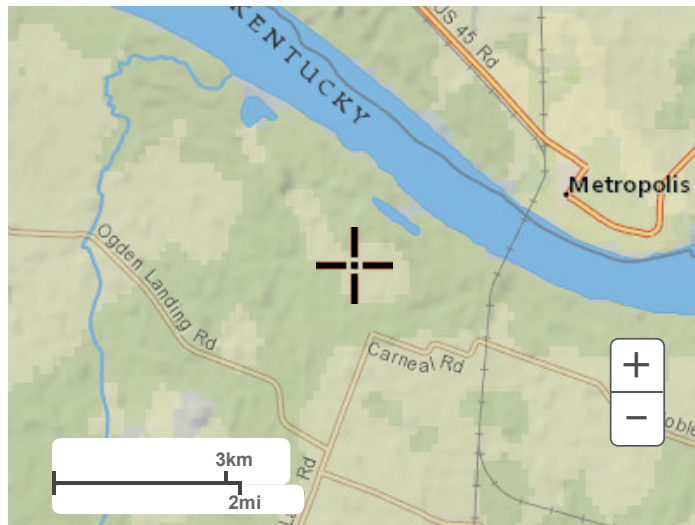


Duration	
5-min	2-day
10-min	3-day
15-min	4-day
30-min	7-day
60-min	10-day
2-hr	20-day
3-hr	30-day
6-hr	45-day
12-hr	60-day
24-hr	

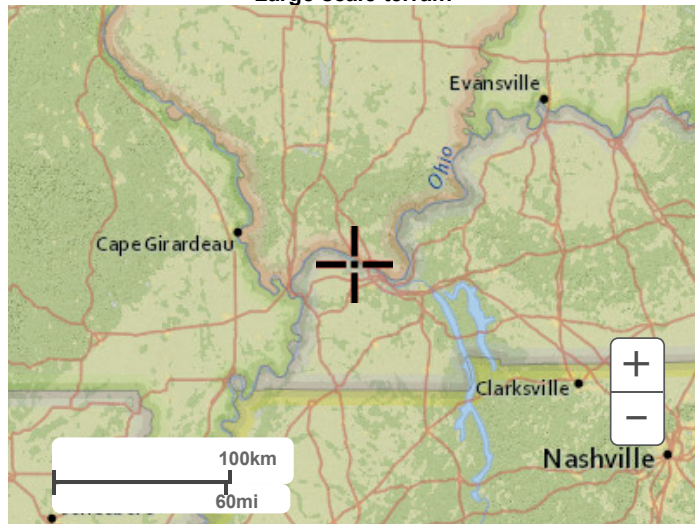
[Back to Top](#)

Maps & aerials

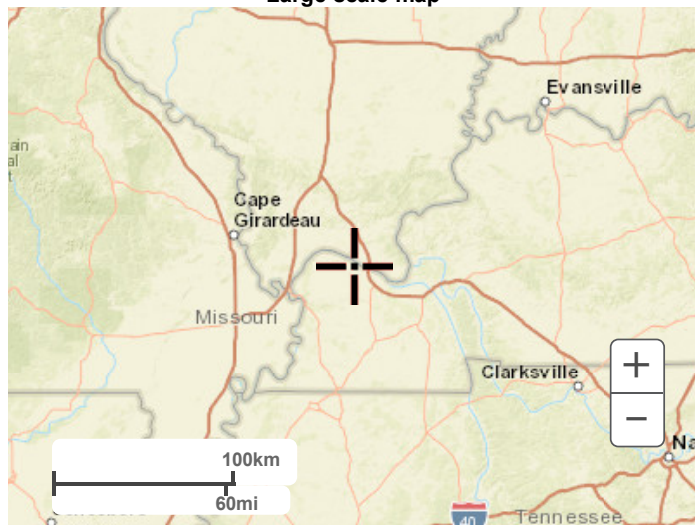
Small scale terrain



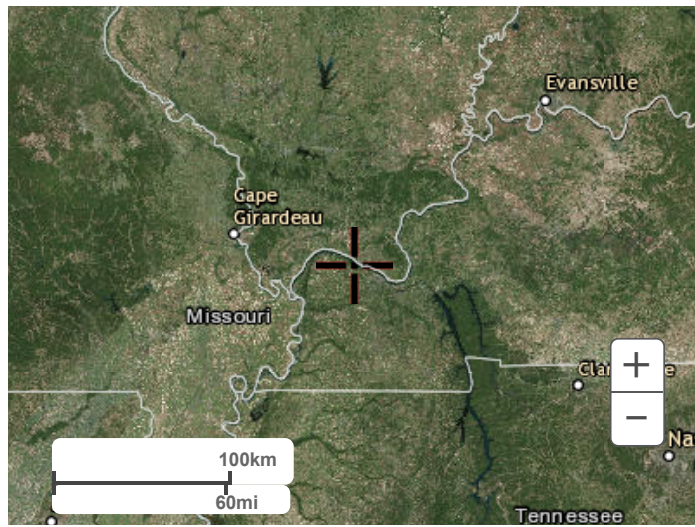
Large scale terrain



Large scale map



Large scale aerial



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[US Department of Commerce](#)
[National Oceanic and Atmospheric Administration](#)
[National Weather Service](#)
[National Water Center](#)
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

[Disclaimer](#)

APPENDIX B
PC-SWMM INPUT FILE

SHAWNEE FOSSIL PLANT PC-SWMM

[OPTIONS]

```

;;Options          Value
;;-----
FLOW_UNITS        CFS
INFILTRATION      CURVE_NUMBER
FLOW_ROUTING      DYNWAVE
START_DATE        11/18/2016
START_TIME        00:00:00
REPORT_START_DATE 11/18/2016
REPORT_START_TIME 00:00:00
END_DATE          11/20/2016
END_TIME          00:00:00
SWEEP_START       01/01
SWEEP_END         12/31
DRY_DAYS          0
REPORT_STEP       00:01:00
WET_STEP          00:05:00
DRY_STEP          00:05:00
ROUTING_STEP      5
ALLOW_PONDING     NO
INERTIAL_DAMPING  PARTIAL
VARIABLE_STEP     0.75
LENGTHENING_STEP 0
MIN_SURFAREA      0
NORMAL_FLOW_LIMITED BOTH
SKIP_STEADY_STATE NO
FORCE_MAIN_EQUATION H-W
LINK_OFFSETS      DEPTH
MIN_SLOPE         0
MAX_TRIALS        8
HEAD_TOLERANCE    0.005
SYS_FLOW_TOL      5
LAT_FLOW_TOL      5
MINIMUM_STEP      0.5
THREADS           4
    
```

[EVAPORATION]

```

;;Type          Parameters
;;-----
CONSTANT        0.0
DRY_ONLY        NO
    
```

[RAINGAGES]

```

;;          Rain      Time      Snow      Data
;;Name      Type      Intrvl  Catch     Source
;;-----
SCS_24h_Type_II_3.72in CUMULATIVE 0:05    1.0      TIMESERIES SCS_24h_Type_II_3.72in
SCS_24h_Type_II_5.21in CUMULATIVE 0:05    1.0      TIMESERIES SCS_24h_Type_II_5.21in
;25-Year Storm Event
SCS_24h_Type_II_6.08in CUMULATIVE 0:05    1.0      TIMESERIES SCS_24h_Type_II_6.08in
SCS_24h_Type_II_7.43in CUMULATIVE 0:05    1.0      TIMESERIES SCS_24h_Type_II_7.43in
SCS_24h_Type_II_9.08in CUMULATIVE 0:05    1.0      TIMESERIES SCS_24h_Type_II_9.08in
    
```

[SUBCATCHMENTS]

```

;;          Total      Pcnt.      Pcnt.
Curb      Snow
;;Name  Raingage      Outlet      Area      Imperv      Width      Slope      Length
Pack
;;-----
    
```

S1	SCS_24h_Type_II_6.08in	DI-14	9.1642	0	328.283	15.77	0
S10	SCS_24h_Type_II_6.08in	DI-40	9.3767	0	333.973	16.24	0
S11	SCS_24h_Type_II_6.08in	DI-51	3.6687	0	221.342	18.52	0
S12	SCS_24h_Type_II_6.08in	DI-36	3.7858	0	221.951	20.17	0
S13	SCS_24h_Type_II_6.08in	DI-48	8.4764	0	313.174	16.16	0
S14	SCS_24h_Type_II_6.08in	DI-34	8.1437	0	291.967	16.21	0
S15	SCS_24h_Type_II_6.08in	DI-46	3.6164	0	185.112	20.1	0
S16	SCS_24h_Type_II_6.08in	DI-30	3.8669	0	260.746	15.78	0
S17	SCS_24h_Type_II_6.08in	DI-42	3.4645	0	221.281	21.14	0
S18	SCS_24h_Type_II_6.08in	DI-28	8.8979	0	313.586	16.1	0
S19	SCS_24h_Type_II_6.08in	North	3.6043	0	1570.033	10	0
S2	SCS_24h_Type_II_6.08in	DI-27	5.3815	0	263.391	18.56	0
S20	SCS_24h_Type_II_6.08in	South	3.523	0	1534.619	10	0
S3	SCS_24h_Type_II_6.08in	DI-12	2.508	0	220.704	18.96	0
S4	SCS_24h_Type_II_6.08in	DI-23	5.5567	0	352.842	18.43	0
S5	SCS_24h_Type_II_6.08in	DI-8	8.6482	0	314.454	16.17	0
S6	SCS_24h_Type_II_6.08in	DI-21	7.9114	0	288.627	15.83	0
S7	SCS_24h_Type_II_6.08in	DI-6	3.6253	0	250.266	19.71	0
S8	SCS_24h_Type_II_6.08in	DI-17	3.1802	0	191.869	19.5	0
S9	SCS_24h_Type_II_6.08in	DI-2	9.0393	0	319.604	16.19	0

[SUBAREAS]

;;Subcatchment	N-Imperv	N-Perov	S-Imperv	S-Perov	PctZero	RouteTo
PctRouted	-----					
;;	-----					
S1	0.012	0.24	0.05	0.2	0	OUTLET
S10	0.012	0.24	0.05	0.2	0	OUTLET
S11	0.012	0.24	0.05	0.2	0	OUTLET
S12	0.012	0.24	0.05	0.2	0	OUTLET
S13	0.012	0.24	0.05	0.2	0	OUTLET
S14	0.012	0.24	0.05	0.2	0	OUTLET
S15	0.012	0.24	0.05	0.2	0	OUTLET
S16	0.012	0.24	0.05	0.2	0	OUTLET
S17	0.012	0.24	0.05	0.2	0	OUTLET
S18	0.012	0.24	0.05	0.2	0	OUTLET
S19	0.012	0.24	0.05	0.2	0	OUTLET
S2	0.012	0.24	0.05	0.2	0	OUTLET
S20	0.012	0.24	0.05	0.2	0	OUTLET
S3	0.012	0.24	0.05	0.2	0	OUTLET
S4	0.012	0.24	0.05	0.2	0	OUTLET
S5	0.012	0.24	0.05	0.2	0	OUTLET
S6	0.012	0.24	0.05	0.2	0	OUTLET
S7	0.012	0.24	0.05	0.2	0	OUTLET
S8	0.012	0.24	0.05	0.2	0	OUTLET
S9	0.012	0.24	0.05	0.2	0	OUTLET

[INFILTRATION]

;;Subcatchment	CurveNum	HydCon	DryTime
;;	-----		
S1	80	0	14
S10	80	0	14
S11	80	0	14
S12	80	0	14
S13	80	0	14
S14	80	0	14
S15	80	0	14
S16	80	0	14
S17	80	0.5	14
S18	80	0.5	14
S19	98	0.5	14
S2	80	0	14
S20	98	0.5	14

S3	80	0.5	14
S4	80	0	14
S5	80	0	14
S6	80	0	14
S7	80	0.5	14
S8	80	0.5	14
S9	80	0.5	14

[JUNCTIONS]

;; ;;Name	Invert Elev.	Max. Depth	Init. Depth	Surcharge Depth	Ponded Area
J1	362	0	0	0	0
J2	362	0	0	0	0
J30	353	0	0	0	0
J31	353.5	8	0	0	0
J32	353.5	8	0	0	0
MH-1	354.04	10.27	0	0	0
MH-10	355.05	16.42	0	0	0
MH-11	355.1	16.48	0	0	0
MH-13	355.32	10.58	0	0	0
MH-15	354.05	12.89	0	0	0
MH-16	354.16	11.64	0	0	0
MH-18	354.37	16.06	0	0	0
MH-19	354.4	16.01	0	0	0
MH-20	354.49	12.91	0	0	0
MH-22	354.79	14.21	0	0	0
MH-24	355.05	16.43	0	0	0
MH-25	355.1	16.47	0	0	0
MH-26	355.17	10.63	0	0	0
MH-29	354.18	10.91	0	0	0
MH-3	354.28	13.32	0	0	0
MH-31	354.37	11.89	0	0	0
MH-32	354.41	12.71	0	0	0
MH-33	354.6	11.5	0	0	0
MH-35	354.87	11.33	0	0	0
MH-37	355.12	13.6	0	0	0
MH-38	355.16	12.16	0	0	0
MH-39	355.19	11.14	0	0	0
MH-4	354.37	16.06	0	0	0
MH-41	354.05	12.1	0	0	0
MH-43	354.35	11.51	0	0	0
MH-44	354.37	12.03	0	0	0
MH-45	354.42	12.89	0	0	0
MH-47	354.69	11.31	0	0	0
MH-49	355.04	11.76	0	0	0
MH-5	354.4	16.01	0	0	0
MH-50	355.12	13.6	0	0	0
MH-51	355.16	12.24	0	0	0
MH-7	354.61	13.39	0	0	0
MH-9	354.95	13.75	0	0	0

[OUTFALLS]

;; ;;Name	Invert Elev.	Outfall Type	Stage/Table Time Series	Tide Gate Route To
O1	352	FREE		NO

[STORAGE]

;; ;;Name	Invert Elev.	Max. Depth	Init. Depth	Storage Curve	Curve Params	Ponded	Evap.
DI-12	355.17	10.63	0	TABULAR	SU3	0	0

DI-14	355.44	10.96	0	TABULAR	SU1	0	0
DI-17	354.27	13.33	0	TABULAR	SU8	0	0
DI-2	354.16	11.64	0	TABULAR	SU9	0	0
DI-21	354.6	13.4	0	TABULAR	SU6	0	0
DI-23	354.94	13.76	0	TABULAR	SU4	0	0
DI-27	355.31	10.59	0	TABULAR	SU2	0	0
DI-28	354.05	12.1	0	TABULAR	SU18	0	0
DI-30	354.35	11.45	0	TABULAR	SU16	0	0
DI-34	354.69	11.31	0	TABULAR	SU14	0	0
DI-36	355.04	11.76	0	TABULAR	SU12	0	0
DI-40	355.38	10.82	0	TABULAR	SU10	0	0
DI-42	354.18	10.91	0	TABULAR	SU17	0	0
DI-46	354.6	11.5	0	TABULAR	SU15	0	0
DI-48	354.87	11.33	0	TABULAR	SU13	0	0
DI-51	355.2	11	0	TABULAR	SU11	0	0
DI-6	354.49	12.91	0	TABULAR	SU7	0	0
DI-8	354.79	14.21	0	TABULAR	SU5	0	0
North	349	15	5	TABULAR	North	0	0
South	349	15	5	TABULAR	South	0	0

[CONDUITS]

;;	Inlet	Outlet		Manning	Inlet	Outlet	Init.	Max.
;;Name	Node	Node	Length	N	Offset	Offset	Flow	Flow
;;	-----							
C1	J1	J30	170	0.013	0	0	0	0
C2	J2	J30	54	0.013	0	0	0	0
Ditch	J30	O1	475.71	0.011	0	0	0	0
Pipe1	MH-1	South	121	0.011	0	5	0	0
Pipe10	MH-10	MH-9	257	0.011	0	0	0	0
Pipe11	MH-11	MH-10	145	0.011	0	0	0	0
Pipe12	DI-12	MH-11	172	0.011	0	0	0	0
Pipe13	MH-13	DI-12	403	0.011	0	0	0	0
Pipe14	DI-14	MH-13	320	0.011	0	0	0	0
Pipe15	MH-15	South	111	0.011	0	5	0	0
Pipe16	MH-16	MH-15	302	0.011	0	0	0	0
Pipe17	DI-17	MH-16	300	0.011	0	0	0	0
Pipe18	MH-18	DI-17	260	0.011	0	0	0	0
Pipe19	MH-19	MH-18	85	0.011	0	0	0	0
Pipe2	DI-2	MH-1	312	0.011	0	0	0	0
Pipe20	MH-20	MH-19	243	0.011	0	0	0	0
Pipe21	DI-21	MH-20	300	0.011	0	0	0	0
Pipe22	MH-22	DI-21	501	0.011	0	0	0	0
Pipe23	DI-23	MH-22	425	0.011	0	0	0	0
Pipe24	MH-24	DI-23	280	0.011	0	0	0	0
Pipe25	MH-25	MH-24	153	0.011	0	0	0	0
Pipe26	MH-26	MH-25	178	0.011	0	0	0	0
Pipe27	DI-27	MH-26	383	0.011	0	0	0	0
Pipe28	DI-28	North	128	0.011	0	5	0	0
Pipe29	MH-29	DI-28	378	0.011	0	0	0	0
Pipe3	MH-3	DI-2	320	0.011	0	0	0	0
Pipe30	DI-30	MH-29	441	0.011	0	0	0	0
Pipe31	MH-31	DI-30	61	0.011	0	0	0	0
Pipe32	MH-32	MH-31	98	0.011	0	0	0	0
Pipe33	MH-33	MH-32	484	0.011	0	0	0	0
Pipe34	DI-34	MH-33	251	0.011	0	0	0	0
Pipe35	MH-35	DI-34	485	0.011	0	0	0	0
Pipe36	DI-36	MH-35	473	0.011	0	0	0	0
Pipe37	MH-37	DI-36	219	0.011	0	0	0	0
Pipe38	MH-38	MH-37	89	0.011	0	0	0	0
Pipe39	MH-39	MH-38	72	0.011	0	0	0	0
Pipe4	MH-4	MH-3	237	0.011	0	0	0	0
Pipe40	DI-40	MH-39	527	0.011	0	0	0	0
Pipe41	MH-41	North	119	0.011	0	5	0	0

Pipe42	DI-42	MH-41	348	0.011	0	0	0	0
Pipe43	MH-43	DI-42	457	0.011	0	0	0	0
Pipe44	MH-44	MH-43	68	0.011	0	0	0	0
Pipe45	MH-45	MH-44	111	0.011	0	0	0	0
Pipe46	DI-46	MH-45	503	0.011	0	0	0	0
Pipe47	MH-47	DI-46	231	0.011	0	0	0	0
Pipe48	DI-48	MH-47	506	0.011	0	0	0	0
Pipe49	MH-49	DI-48	455	0.011	0	0	0	0
Pipe5	MH-5	MH-4	78	0.011	0	0	0	0
Pipe50	MH-50	MH-49	220	0.011	0	0	0	0
Pipe51	MH-51	MH-50	96	0.011	0	0	0	0
Pipe52	DI-51	MH-51	96	0.011	0	0	0	0
Pipe53	J32	J30	184.84	0.011	0	0	0	0
Pipe54	J31	J30	192.81	0.011	0	0	0	0
Pipe6	DI-6	MH-5	239	0.011	0	0	0	0
Pipe7	MH-7	DI-6	320	0.011	0	0	0	0
Pipe8	DI-8	MH-7	481	0.011	0	0	0	0
Pipe9	MH-9	DI-8	446	0.011	0	0	0	0

[ORIFICES]

;;	Inlet	Outlet	Orifice	Crest	Disch.	Flap	
Open/Close							
;;Name	Node	Node	Type	Height	Coeff.	Gate	Time
;;	-----	-----	-----	-----	-----	-----	-----
N-spillway	North	J31	BOTTOM	10	0.65	NO	0
OR1	South	J32	SIDE	5	0.65	NO	0
OR10	North	J31	SIDE	5	0.65	NO	0
OR11	North	J31	SIDE	5	0.65	NO	0
OR12	North	J31	SIDE	5	0.65	NO	0
OR2	South	J32	SIDE	5	0.65	NO	0
OR3	South	J32	SIDE	5	0.65	NO	0
OR4	South	J32	SIDE	5	0.65	NO	0
OR5	South	J32	SIDE	5	0.65	NO	0
OR6	South	J32	SIDE	5	0.65	NO	0
OR7	North	J31	SIDE	5	0.65	NO	0
OR8	North	J31	SIDE	5	0.65	NO	0
OR9	North	J31	SIDE	5	0.65	NO	0
S-spillway	South	J32	BOTTOM	10	0.65	NO	0

[WEIRS]

;;	Inlet	Outlet	Weir	Crest	Disch.	Flap	End	End	
;;Name	Node	Node	Type	Height	Coeff.	Gate	Con.	Coeff.	
Surcharge	RoadWidth	RoadSurf							
;;	-----	-----	-----	-----	-----	-----	-----	-----	
N-emergency	North	J1	TRAPEZOIDAL	13	3.33	NO	0	0	YES
S-emergency	South	J2	TRAPEZOIDAL	13	3.33	NO	0	0	YES

[XSECTIONS]

;;Link	Shape	Geom1	Geom2	Geom3	Geom4
Barrels					
;;	-----	-----	-----	-----	-----
C1	TRAPEZOIDAL	2	10	3	1
C2	TRAPEZOIDAL	2	10	3	1
Ditch	TRAPEZOIDAL	5	5	3	1
Pipe1	CIRCULAR	3.739	0	0	1
Pipe10	CIRCULAR	3.272	0	0	1
Pipe11	CIRCULAR	3.272	0	0	1
Pipe12	CIRCULAR	3.272	0	0	1
Pipe13	CIRCULAR	3.272	0	0	1
Pipe14	CIRCULAR	3.272	0	0	1

Pipe15	CIRCULAR	3.272	0	0	0	1
Pipe16	CIRCULAR	3.272	0	0	0	1
Pipe17	CIRCULAR	3.272	0	0	0	1
Pipe18	CIRCULAR	3.272	0	0	0	1
Pipe19	CIRCULAR	3.272	0	0	0	1
Pipe2	CIRCULAR	3.739	0	0	0	1
Pipe20	CIRCULAR	3.272	0	0	0	1
Pipe21	CIRCULAR	3.272	0	0	0	1
Pipe22	CIRCULAR	3.272	0	0	0	1
Pipe23	CIRCULAR	3.272	0	0	0	1
Pipe24	CIRCULAR	3.272	0	0	0	1
Pipe25	CIRCULAR	3.272	0	0	0	1
Pipe26	CIRCULAR	3.272	0	0	0	1
Pipe27	CIRCULAR	3.272	0	0	0	1
Pipe28	CIRCULAR	3.739	0	0	0	1
Pipe29	CIRCULAR	3.739	0	0	0	1
Pipe3	CIRCULAR	3.739	0	0	0	1
Pipe30	CIRCULAR	3.272	0	0	0	1
Pipe31	CIRCULAR	3.272	0	0	0	1
Pipe32	CIRCULAR	3.272	0	0	0	1
Pipe33	CIRCULAR	3.272	0	0	0	1
Pipe34	CIRCULAR	3.272	0	0	0	1
Pipe35	CIRCULAR	3.272	0	0	0	1
Pipe36	CIRCULAR	3.272	0	0	0	1
Pipe37	CIRCULAR	3.272	0	0	0	1
Pipe38	CIRCULAR	3.272	0	0	0	1
Pipe39	CIRCULAR	3.272	0	0	0	1
Pipe4	CIRCULAR	3.739	0	0	0	1
Pipe40	CIRCULAR	3.272	0	0	0	1
Pipe41	CIRCULAR	3.272	0	0	0	1
Pipe42	CIRCULAR	3.272	0	0	0	1
Pipe43	CIRCULAR	3.272	0	0	0	1
Pipe44	CIRCULAR	3.272	0	0	0	1
Pipe45	CIRCULAR	3.272	0	0	0	1
Pipe46	CIRCULAR	3.272	0	0	0	1
Pipe47	CIRCULAR	3.272	0	0	0	1
Pipe48	CIRCULAR	3.272	0	0	0	1
Pipe49	CIRCULAR	3.272	0	0	0	1
Pipe5	CIRCULAR	3.739	0	0	0	1
Pipe50	CIRCULAR	3.272	0	0	0	1
Pipe51	CIRCULAR	3.272	0	0	0	1
Pipe52	CIRCULAR	3.272	0	0	0	1
Pipe53	CIRCULAR	2	0	0	0	1
Pipe54	CIRCULAR	2	0	0	0	1
Pipe6	CIRCULAR	3.739	0	0	0	1
Pipe7	CIRCULAR	3.739	0	0	0	1
Pipe8	CIRCULAR	3.739	0	0	0	1
Pipe9	CIRCULAR	3.739	0	0	0	1
N-spillway	CIRCULAR	2	0	0	0	
OR1	CIRCULAR	0.125	0	0	0	
OR10	CIRCULAR	0.125	0	0	0	
OR11	CIRCULAR	0.125	0	0	0	
OR12	CIRCULAR	0.125	0	0	0	
OR2	CIRCULAR	0.125	0	0	0	
OR3	CIRCULAR	0.125	0	0	0	
OR4	CIRCULAR	0.125	0	0	0	
OR5	CIRCULAR	0.125	0	0	0	
OR6	CIRCULAR	0.125	0	0	0	
OR7	CIRCULAR	0.125	0	0	0	
OR8	CIRCULAR	0.125	0	0	0	
OR9	CIRCULAR	0.125	0	0	0	
S-spillway	CIRCULAR	2	0	0	0	
N-emergency	TRAPEZOIDAL	2	10	5	5	

SU10		7.62	5027.48
SU10		8.62	9538.41
SU10		9.62	13779.39
SU10		10.62	17849.18
SU10		10.82	18671.25
SU11	Storage	0	38.5
SU11		6.02	38.5
SU11		6.8	1551.96
SU11		7.8	4552.06
SU11		8.8	8582.74
SU11		9.8	12048.12
SU11		10.8	15579.32
SU11		11	16277.03
SU12	Storage	0	38.5
SU12		6.76	38.5
SU12		6.96	355.59
SU12		7.96	3089.71
SU12		8.96	7281.46
SU12		9.96	10876.5
SU12		10.96	14385.72
SU12		11.76	17265.86
SU13	Storage	0	38.5
SU13		6.37	38.5
SU13		7.13	1476.29
SU13		8.13	4589.11
SU13		9.13	8889.86
SU13		10.13	12651.27
SU13		11.13	16401.38
SU13		11.33	17092.52
SU14	Storage	0	38.5
SU14		6.33	38.5
SU14		7.31	2424.7
SU14		8.31	5402.92
SU14		9.31	8668.28
SU14		10.31	11629.54
SU14		11.31	14595.13
SU15	Storage	0	38.5
SU15		6.33	38.5
SU15		6.4	134.76
SU15		7.4	2583.63
SU15		8.4	5527.2
SU15		9.4	9011.89
SU15		10.4	12361.01
SU15		11.4	15733.36
SU15		11.5	16461.34
SU16	Storage	0	38.5
SU16		6.52	38.5
SU16		6.65	344.78
SU16		7.65	2891.95
SU16		8.65	6863.96
SU16		9.65	11256.31
SU16		10.65	15386.52
SU16		11.45	18683.21
SU17	Storage	0	38.5
SU17		5.96	38.5
SU17		6.82	1561.06

SU17		7.82	4512.1
SU17		8.82	8548.05
SU17		9.82	12043.83
SU17		10.82	15536.9
SU17		10.96	15849.29
SU18	Storage	0	38.5
SU18		7.11	38.5
SU18		7.95	1965.46
SU18		8.95	5740.29
SU18		9.95	9430.91
SU18		10.95	12814.43
SU18		11.95	16188.36
SU18		12.15	16886.33
SU2	Storage	0	38.5
SU2		5.56	38.5
SU2		5.69	263.16
SU2		6.69	2906.41
SU2		7.69	6442.56
SU2		8.69	9296.94
SU2		9.69	12168.04
SU2		10.59	14759.17
SU3	Storage	0	38.5
SU3		5.62	38.5
SU3		5.83	251.33
SU3		6.83	2075.3
SU3		7.83	4715
SU3		8.83	7249.66
SU3		9.83	10025.45
SU3		10.63	12279.74
SU4	Storage	0	38.5
SU4		8.79	38.5
SU4		9.06	409.15
SU4		10.06	2762.28
SU4		11.06	6462
SU4		12.06	11146.58
SU4		13.06	16286.26
SU4		13.76	19401.77
SU5	Storage	0	38.5
SU5		9.23	38.5
SU5		10.21	2376.15
SU5		11.21	6372.16
SU5		12.21	10875.9
SU5		13.21	14826.39
SU5		14.22	18814.42
SU6	Storage	0	38.5
SU6		8.4	38.5
SU6		9.4	2112.13
SU6		10.4	4613.06
SU6		11.4	7604.39
SU6		12.4	10785.09
SU6		13.38	13677.33
SU7	Storage	0	38.5
SU7		7.91	38.5
SU7		8.51	1192.78
SU7		9.51	4301.47
SU7		10.51	7893.57

SU7		11.51	11887.75
SU7		12.51	15600.63
SU7		12.91	17120.95
SU8	Storage	0	38.5
SU8		8.31	38.5
SU8		8.73	799.25
SU8		9.73	3785.41
SU8		10.73	7512.08
SU8		11.73	11613.82
SU8		12.73	15403.11
SU8		13.33	17663.26
SU9	Storage	0	38.5
SU9		6.67	38.5
SU9		6.84	308.12
SU9		7.84	2230.74
SU9		8.84	5253.25
SU9		9.84	8803.95
SU9		10.84	12090.59
SU9		11.64	14662.58

**APPENDIX C
BENCH DITCH AND FLUME
CALCULATIONS**

Bench Ditch Hydraulic Capacity



Client	Tennessee Valley Authority
Project Name	SHF Proposed CCR Landfill
Location	Bench ditch sizing calculation
Date	Paducah, McCracken County, KY
Stantec Project Number	3/7/2017
	175666001

Prepared By:	Caitlin Johnson, EIT
Reviewed By:	Joshua Kopp, PE

Drainage Ditch - Channel Capacity & Depth Calculations

Channel Section 4
4. Non-Symmetrical Triangular Section

Input Data:

Manning's "n" value	0.033
Longitudinal Slope - S_o	0.020 ft/ft
Design Discharge - Q	5.9 ft ³ /s - cfs

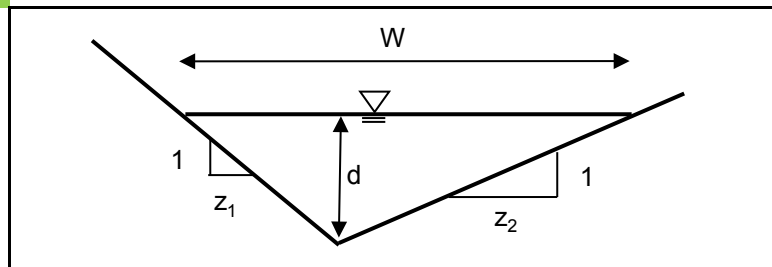
Channel Geometry Data:

Side Slope(s)	
z_1 or z	10.0 z H:1V
z_2	4.0 z H:1V

Output Data:

Calculated Depth - d	0.56 feet
Calculated Top Width - W	7.82 feet
Calculated Area - A	2.18 ft ²
Calc. Wetted Perimeter - Wp	7.92 feet
Calc. Hydr. Radius - R	0.28 feet
Calculated Discharge - Q'	5.91 ft ³ /s - cfs
Convergence	0.0400 ft ³ /s - cfs
Calculated Velocity	2.71 ft / s
Calculated Shear Stress - τ_d	0.70 lb / ft ²

Channel Sketch



Governing Geometry Equations

$$W = d(z_1 + z_2)$$

$$A = \frac{d^2}{2} (z_1 + z_2)$$

$$W_p = d(\sqrt{z_1^2 + 1} + \sqrt{z_2^2 + 1})$$

$$Q = \frac{1.49}{n} AR^{\frac{2}{3}} S_o^{\frac{1}{2}} \quad R = \frac{A}{W_p}$$

$$\tau_d = \gamma d S \quad V = \frac{Q}{A}$$

Riprap Ditch Hydraulic Capacity



Client	Tennessee Valley Authority
Project Name	SHF Proposed CCR Landfill
	Flume sizing calculation
Location	Paducah, McCracken County, KY
Date	3/6/2017
Stantec Project Number	175666001

Prepared By:	Caitlin Johnson, EIT
Reviewed By:	Joshua Kopp, PE

Drainage Ditch - Channel Capacity & Depth Calculations

Channel Section 1
 1. Uniform (Symmetrical) Trapezoidal Section

Input Data:

Manning's "n" value	0.04
Longitudinal Slope - S_o	0.150 ft/ft
Design Discharge - Q	27.5 ft ³ /s - cfs

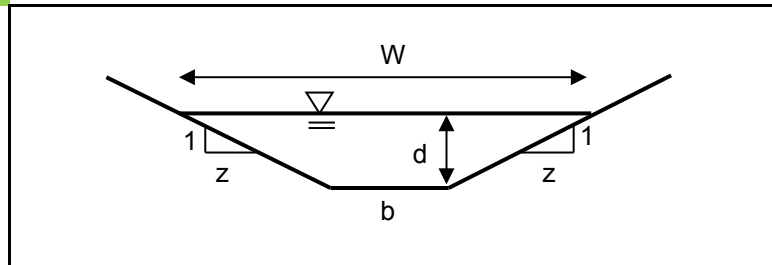
Channel Geometry Data:

Bottom Width(s) b_1 or b	6 feet
Side Slope(s) z_1 or z	3.0 z H:1V

Output Data:

Calculated Depth - d	0.48 feet
Calculated Top Width - W	8.88 feet
Calculated Area - A	3.56 ft ²
Calc. Wetted Perimeter - W_p	9.03 feet
Calc. Hydr. Radius - R	0.39 feet
Calculated Discharge - Q'	27.67 ft ³ /s - cfs
Convergence	0.1682 ft ³ /s - cfs
Calculated Velocity	7.76 ft / s
Calculated Shear Stress - τ_d	4.49 lb / ft ²

Channel Sketch



Governing Geometry Equations

$$W = b + 2dz$$

$$A = bd + zd^2$$

$$W_p = b + 2d\sqrt{z^2 + 1}$$

$$Q = \frac{1.49}{n} AR^{\frac{2}{3}} S_o^{\frac{1}{2}} \quad R = \frac{A}{W_p}$$

$$\tau_d = \gamma dS \quad V = \frac{Q}{A}$$

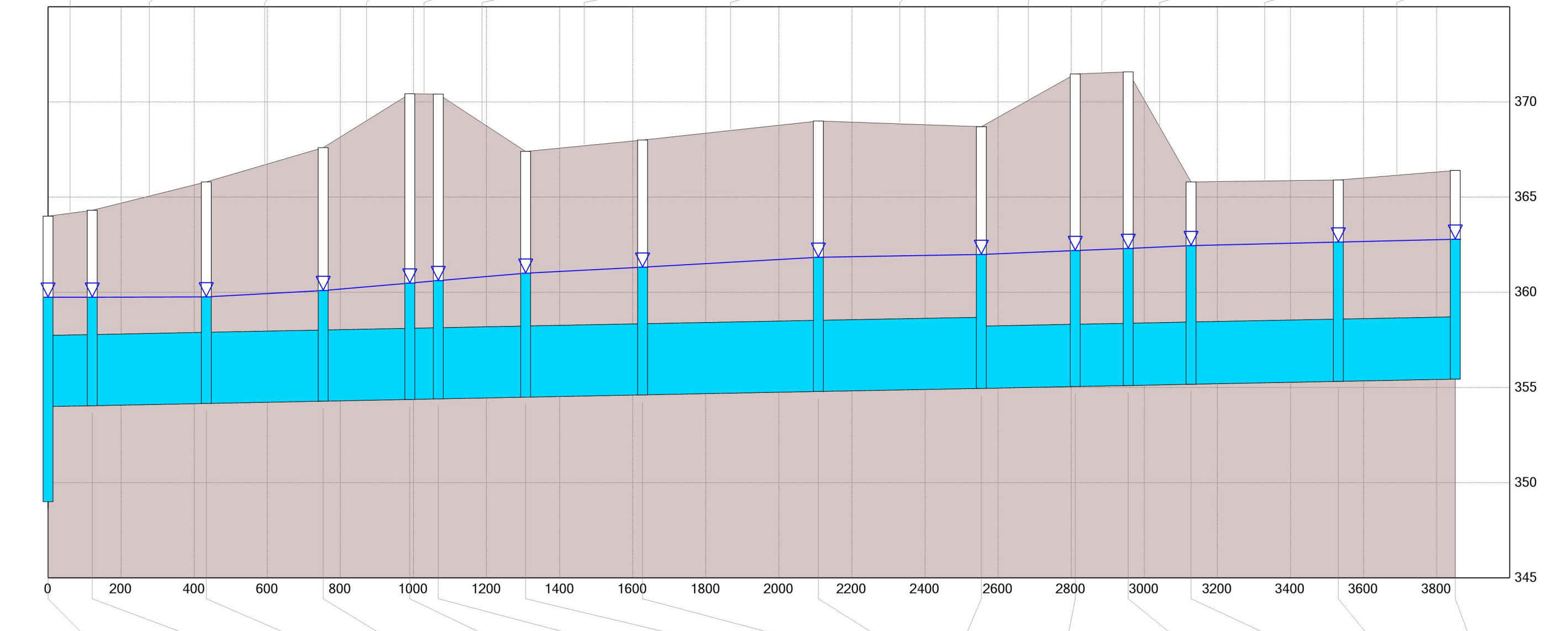
APPENDIX D
25-YEAR, 24-HOUR STORM EVENT
PROFILES

SHAWNEE FOSSIL PLANT PC-SWMM

STORM SEWER 1 (SS1) 25-YR STORM EVENT

Links:	Pipe1	Pipe2	Pipe3	Pipe4	Pipe5	Pipe6	Pipe7	Pipe8	Pipe9	Pipe10	Pipe11	Pipe12	Pipe13	Pipe14
Q=	81.373 cfs	81.369 cfs	57.769 cfs	57.75 cfs	57.714 cfs	57.672 cfs	48.816 cfs	48.801 cfs	30.765 cfs	30.782 cfs	30.771 cfs	30.757 cfs	22.204 cfs	22.184 cfs
L=	121 ft	312 ft	320 ft	237 ft	78 ft	239 ft	320 ft	481 ft	446 ft	257 ft	145 ft	172 ft	403 ft	320 ft
D=	3.739 ft	3.739 ft	3.739 ft	3.739 ft	3.739 ft	3.739 ft	3.739 ft	3.739 ft	3.739 ft	3.272 ft	3.272 ft	3.272 ft	3.272 ft	3.272 ft
V=	8.013 ft/s	7.411 ft/s	5.261 ft/s	5.26 ft/s	5.256 ft/s	5.252 ft/s	4.446 ft/s	4.445 ft/s	2.802 ft/s	3.661 ft/s	3.659 ft/s	3.658 ft/s	2.641 ft/s	2.638 ft/s
S=	0.00033 ft/ft	0.00038 ft/ft	0.00038 ft/ft	0.00038 ft/ft	0.00038 ft/ft	0.00038 ft/ft	0.00038 ft/ft	0.00037 ft/ft	0.00036 ft/ft	0.00039 ft/ft	0.00034 ft/ft	0.00041 ft/ft	0.00037 ft/ft	0.00038 ft/ft

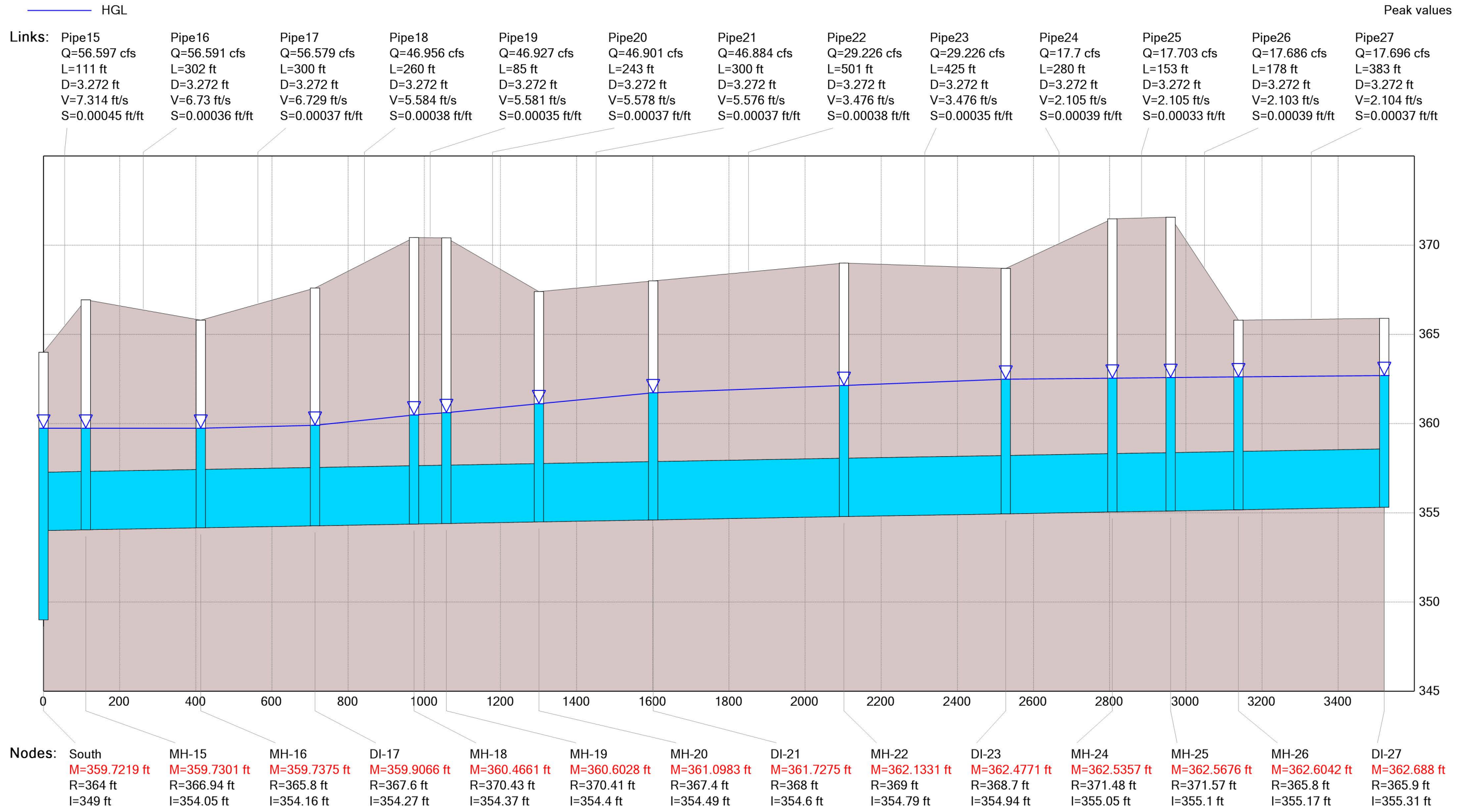
Peak values



Nodes:	South	MH-1	DI-2	MH-3	MH-4	MH-5	DI-6	MH-7	DI-8	MH-9	MH-10	MH-11	DI-12	MH-13	DI-14
M=	359.7219 ft	359.7336 ft	359.743 ft	360.0867 ft	360.48 ft	360.6007 ft	360.9911 ft	361.3032 ft	361.826 ft	361.9857 ft	362.1806 ft	362.295 ft	362.4368 ft	362.6261 ft	362.7764 ft
R=	364 ft	364.31 ft	365.8 ft	367.6 ft	370.43 ft	370.41 ft	367.4 ft	368 ft	369 ft	368.7 ft	371.47 ft	371.58 ft	365.8 ft	365.9 ft	366.4 ft
I=	349 ft	354.04 ft	354.16 ft	354.28 ft	354.37 ft	354.4 ft	354.49 ft	354.61 ft	354.79 ft	354.95 ft	355.05 ft	355.1 ft	355.17 ft	355.32 ft	355.44 ft

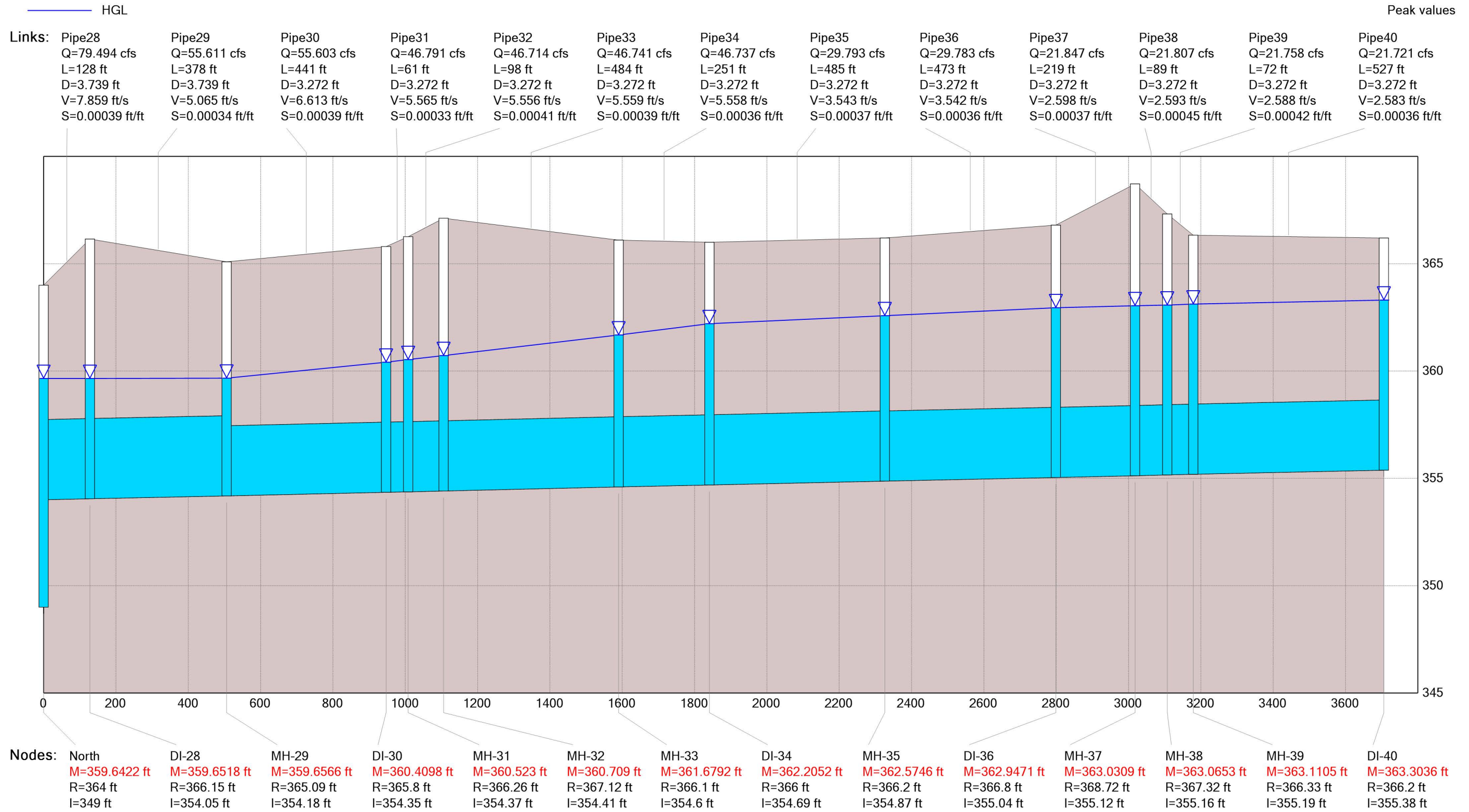
SHAWNEE FOSSIL PLANT PC-SWMM

STORM SEWER 2 (SS2) 25-YR STORM EVENT



SHAWNEE FOSSIL PLANT PC-SWMM

STORM SEWER 3 (SS3) 25-YR STORM EVENT



SHAWNEE FOSSIL PLANT PC-SWMM

STORM SEWER 4 (SS4) 25-YR STORM EVENT

